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**Title:** Identification of most significant factors for modeling deterioration of sewer pipes

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20 **Abstract**

21 Existing methods used to identify the important factors that can improve predicting structural  
22 deterioration of sewer pipes rarely take into account the interactions and correlations among  
23 them. Here we present a standardized method that combines use of the Cox model and  
24 likelihood ratio test, and overcomes these limitations of previously employed methods. This  
25 combined method is applied to the pipes of two Canadian sewer systems, and its results are  
26 compared to the results of two simpler methods for the identification of the factors that  
27 significantly influence sewer pipe deterioration. The three methods identified pipe age as the  
28 principal factor driving the structural deterioration of sewer pipes. However, slight differences  
29 between the methods for other potential influential factors (material, slope and diameter)  
30 showed that accounting for the interactions and correlations among factors, as is possible with  
31 the proposed method, is crucial to identifying the factors having a significant impact on pipe  
32 deterioration.

33

34 **Keywords:** covariates; Cox model; Kruskal-Wallis; likelihood ratio; structural state; survival  
35 analysis

36

37 **Résumé**

38 Les méthodes existantes permettant d'identifier les facteurs d'influence qui doivent être pris en  
39 compte dans la modélisation de la détérioration structurale des conduites d'égout prennent  
40 rarement en compte les interactions et/ou les corrélations entre ces facteurs. Une méthode  
41 standardisée, basée sur l'utilisation combinée du modèle de Cox et du test du rapport de  
42 vraisemblance, est proposée dans cet article. Cette méthode est appliquée aux conduites de  
43 deux réseaux d'égout canadiens et ses résultats sont comparés aux résultats de deux  
44 méthodes plus simples pour l'identification des principaux facteurs influents. Les trois méthodes  
45 identifient l'âge des conduites comme étant le principal facteur d'influence dans le processus de  
46 détérioration des conduites. Cependant, de légères différences entre les résultats de ces  
47 méthodes concernant certains facteurs potentiellement influents (matériau, pente et diamètre)  
48 démontrent que la prise en compte des interactions et des corrélations entre les facteurs,  
49 rendue possible avec la méthode proposée, est cruciale pour identifier les facteurs ayant un  
50 impact significatif.

51

52 **Mots-clés:** analyse de survie; covariables; état structural; Kruskal-Wallis; modèle de Cox;  
53 rapport de vraisemblance.

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## 57 **1. Introduction**

58 Many mathematical models exist to predict the structural condition of sewer pipes over time,  
59 depending on several variables. These models can be classified into three groups (Ana and  
60 Bauwens, 2010): 1) physical models that are based on the physical mechanisms governing the  
61 deterioration of pipes (e.g. Konig, 2005); 2) artificial intelligence models (e.g. Tran et al. 2006;  
62 Kleiner et al. 2006); and 3) statistical models (e.g. Duchesne et al. 2013). The input data for  
63 each of the three model types are pipe condition ratings, which summarize the defects (nature,  
64 number, and severity) observed in sewer pipes during televisual inspection. Statistical models,  
65 however, remain the most commonly used method to predict the structural condition of sewer  
66 pipes (Duchesne et al. 2013).

67  
68 The principal classes of statistical models that have been applied to the structural deterioration  
69 of sewer pipes are: 1) survival models, 2) Markovian models, 3) regression models, and  
70 4) classification models. In survival models, the process of pipe deterioration is represented by  
71 the successive transition from one condition state to another (Ana and Bauwens, 2010; Baur et  
72 al., 2004). The period of time during which the pipes remain in a given structural state is  
73 considered a random variable, described by different distribution functions (e.g. Weibull,  
74 exponential, or Hertz), thus defining the process of sewer pipe deterioration over time (Horold  
75 and Baur 1999; Mailhot et al. 2000). The result of the survival model is the proportion of sewer  
76 lines in a given structural state according to age (Baur and Herz 2002; Duchesne et al. 2013;  
77 Ugarelli et al. 2013). Markovian models describe the discrete-time stochastic process whereby  
78 the transition probability to the following state class depends only on the current state (Baik et  
79 al. 2006; Ross 2000; Wirahadikusumah et al. 2001). This type of model gives the probability that  
80 a pipe moves from one condition state to another over a given time interval (Baik et al., 2006;  
81 Duchesne et al. 2013; Micevski et al. 2002; Ugarelli et al. 2013). This transition probability is

82 constant over time, although in semi-Markovian models (also known as non-homogenous  
83 Markov models), the transition probability varies with time (Ana and Bauwens 2010). In  
84 regression models, the probability that a pipe is in a given state after a given period of time is  
85 evaluated according to multiple independent variables (Ana et al. 2009; Ana et al. 2008;  
86 Ariaratnam et al. 2001; Baik et al. 2006; Chughtai and Zayed 2008; Salman and Salem 2012;  
87 Younis and Knight 2010). Regression models can also be used to determine the transition  
88 probabilities of Markov models (Baik et al. 2006; Le Gat 2008). Finally, different classification  
89 models exist; an example of such a model based on a Random Forest Approach is given in  
90 Harvey and McBean (2014).

91  
92 Variables included in statistical models designed to predict the structural integrity of sewer pipes  
93 may be specific to the pipes themselves, such as the age, time of installation, size, length,  
94 shape, material, network type, slope, burial depth, and hydraulic performance. Variables may  
95 also relate to the environment where the pipes are found, including the soil type, location, level  
96 of traffic in close proximity, type of pipe bedding, temperature, and freezes. Several studies  
97 have been conducted over the past fifteen years to determine which factors (i.e. independent  
98 variables) should be incorporated into models predicting the structural deterioration of sewer  
99 pipes. Table 1 provides a summary of these studies and their results.

100

101 ➤ **Insert Table 1 here**

102

103 The factor that was most frequently identified as having an impact on the structural deterioration  
104 of sewer pipes was age, followed by pipe diameter, pipe length, pipe material, network type and  
105 pipe slope (Table 1). There was no consensus, however, concerning the method that should be  
106 applied to identify the factors that significantly affect sewer pipe deterioration. Also, the factors  
107 identified as influential varied greatly among studies because: 1) differences in how factors

108 influence the structural deterioration of sewer pipes mainly depended on the networks where the  
109 studies were conducted, and 2) most studies were ad hoc and used different approaches and  
110 evaluated different combinations of factors for different networks. Additionally, the interactions  
111 among multiple factors were only evaluated in a few studies and correlations among factors  
112 were rarely taken into account (except in the case studies conducted by Ariaratnam et al. 2001,  
113 and Chughtai and Zayed 2008). In this context, the first objective of the work presented here is  
114 to propose a new, robust and standardized method to identify the most influential factors that  
115 should be retained in the models predicting the structural deterioration of sewer pipes. The  
116 method is based on the likelihood ratio test and the Cox model, used for the first time here to  
117 simulate the structural deterioration process of sewer pipes, which makes it possible to integrate  
118 several impact factors as uncorrelated explanatory variables. Also, given the large variation in  
119 the results found in the literature concerning the influential factors for sewer pipe deterioration,  
120 the second objective is to determine the factors that should be considered for modeling the  
121 structural deterioration of Canadian sewers, as a function of their main characteristics and the  
122 data that are usually available in Canadian municipalities. This information will help guide  
123 network managers to the most appropriate deterioration model for their needs. The third  
124 objective is to compare the results of the proposed method for the identification of the most  
125 influential factors with the results of two simpler methods, one of them often applied in  
126 previously published studies. To attain these objectives, analyses were performed using data  
127 provided by two Canadian municipalities, as described in the following sections.

128

## 129 **2. Methodology**

### 130 **2.1 Case studies**

131 The analyses were performed using data from two different Canadian sewer networks, hereafter  
132 referred to as Network A and Network B, located in the province of Quebec. The data provided

133 by network managers consisted of 1) sewer pipe characteristics (installation date, material,  
134 diameter, length, and location), and 2) observed structural defects in the pipes that were  
135 inspected using a camera, along with the inspection date. The database of the inspected sewer  
136 pipes included 15 years of inspections for Network A (1998 to 2012), and 3 years of inspections  
137 for Network B (2003 to 2005). In both databases, a “pipe” is defined as a portion of the sewer  
138 network located between two manholes or adjacent street junctions, with a constant slope,  
139 diameter, and material.

140

141 For the analyses presented in this paper, only the results from inspections performed with a  
142 zoom camera were retained, as these were the most common type. For Network A, all the  
143 observed defects were categorized using the WRc (1994) system, on a scale of 1 to 5. For  
144 Network B, the defects were originally characterized using the CERIU (2004) system, thus  
145 CERIU grades were converted to WRc grades using the conversion table presented in  
146 Duchesne *et al.* (2011). The highest grade for a structural pipe defect was retained to quantify  
147 its overall structural state. Another state, state 0, was incorporated into our analyses for pipes  
148 without noted defects. Consequently, a pipe in state 0 would have no significant observable  
149 structural defects, while a pipe in state 5 would need immediate intervention. Because there  
150 were fewer pipes in the worst deterioration states, condition states 2 and 3, and condition states  
151 4 and 5, were grouped together. In summary, the inspected pipes were classified into four  
152 distinct deterioration states: 1) state 0 (no structural defect observed); 2) state 1 (only minor  
153 structural defects observed); 3) state 2-3 (moderate deterioration); and 4) state 4-5 (one or more  
154 severe structural defects observed).

155

156 Table 2 presents the main characteristics of the pipes in Networks A and B. Since there is a  
157 high level of uncertainty in the installation dates of older pipes, “total network” refers to all the  
158 pipes installed in 1900 or later, whereas “inspected pipes” refers to all the pipes that were 70

159 years old or newer at the time of inspection. In Table 2, “other material” includes asbestos  
160 cement, non-reinforced concrete, corrugated steel, brick, cast iron, pipe reinforced with glass  
161 fiber, polyethylene, steel, and vitrified clay. Table 3 gives the proportions of the pipes in states 0,  
162 1, 2-3, and 4-5 (corresponding respectively to very good, good, fair and poor structural state) in  
163 the different age ranges for Networks A and B.

164  
165 ➤ **Insert Table 2 here**

166  
167 ➤ **Insert Table 3 here**

## 168 169 170 **2.2 Estimation of significant influential factors**

171 The applied methodology is summarized in Figure 1. Details are provided in the sections below.

172  
173 ➤ **Insert Figure 1 here**

### 174 175 *2.2.1 Proposition of the Cox method to identify the significant influential factors*

176 For the identification of the influential factors that should be taken into account for predicting the  
177 deterioration state of sewer pipes, we propose use of a Cox model, and then to evaluate the  
178 statistical significance of the model coefficient related to each influential factor (or covariate)  
179 (Figure 1, method A). Only the factors related to the coefficients that have been identified as  
180 statistically significant should then be retained in the model.

#### 181 182 2.2.1.1 Description of the proposed Cox model

183 The Cox model is based on survival analysis principles, which represents the time that a pipe  
184 remains in each deterioration state as a random variable. This model, widely used in medical



185 science to predict the time before an event (death, recidivism of a disease or cure) occurs (Klein  
186 and Moeschberger 2003), has never been used, to our knowledge, for predicting the structural  
187 deterioration of sewer pipes (although it was used to predict breaks in water mains; e.g.  
188 Andreou *et al.*, 1987a and 1987b).

189  
190 The equations of the Cox model for the modeling of sewer pipes deterioration are developed  
191 below for the specific case of four possible structural condition states. However, they could be  
192 developed similarly for any number of structural condition states. When four different structural  
193 condition states are considered, three residence times  $t$  should be modeled. Indeed, once a  
194 pipe has entered the fourth and final state it will remain in this state until it is replaced or  
195 repaired. Consequently, the residence time in the final state does not have to be modeled. The  
196 probability density functions (pdf) of the three residence times,  $f_j(t, X)$ , are expressed in the  
197 proposed model by exponential functions, as suggested by Serpente (1994) and Duchesne *et*  
198 *al.* (2013). Consequently, the pdfs of residence times are expressed as follows (Equation 1):

$$199 \quad f_j(t, X) = k_j e^{\beta_j X} e^{-k_j e^{\beta_j X} t} \quad (1)$$

200 with:  $t$  = residence time in structural state  $j$ ;  $X$  = vector of explanatory variables; and  $k_j$  and  $\beta_j$  =  
201 model parameters corresponding to the structural state  $j$  ( $j = 0, 1$  or  $2$ ). When the  $\beta_j$  coefficients  
202 associated with the model covariates  $X$  equal zero, the Cox model becomes equivalent to the  
203 model of Duchesne *et al.* (2013).

204  
205 The probability that a pipe will remain longer than time  $t$  in any state  $j$  is expressed by the  
206 survival function  $S_j(t, X)$  (Equation 2):

$$207 \quad S_j(t, X) = \Pr(T > t) = \int_t^{\infty} f_j(t, X) dt \quad (2)$$

208 At the moment of inspection, only the physical and functional characteristics, including the  
 209 deterioration state and the age of the pipe are available, but not the time at which it entered its  
 210 current deterioration state and the previous ones. For example, if a pipe in the fourth  
 211 deterioration state is inspected 60 years after its installation, 60 years is the total time the pipe  
 212 was in the first, second, third and fourth states. For this reason the pdf and survival functions for  
 213 cumulative residence times need to be developed.

214  
 215 The equation for  $f_{01}$ , the pdf of the sum of residence times in the first and second states, is given  
 216 in Equation 3:

$$217 \quad f_{01}(t, X) = f_0 * f_1(t, X) = \int_{\tau=-\infty}^{\infty} f_0(\tau, X) f_1(t - \tau, X) d\tau \quad (3)$$

218 where \* is the convolution product. As  $f_0$ ,  $f_1$  and  $f_{01}$  are defined only for positive real numbers,  
 219 then  $\tau \geq 0$  and  $(t - \tau) \geq 0$ , and consequently  $\tau \leq t$ , thus the previous integral can be simplified to:

$$220 \quad f_{01}(t, X) = \int_{\tau=-\infty}^{\infty} f_0(\tau, X) f_1(t - \tau, X) d\tau = \int_{\tau=0}^t f_0(\tau, X) f_1(t - \tau, X) d\tau \quad (4)$$

221  
 222 The corresponding survival function, Equation 5, gives the probability that a pipe will be in the  
 223 second state or lower (i.e., in the first or second state) at time  $T$ :

$$224 \quad S_{01}(T, X) = \int_{t=T}^{+\infty} f_{01}(t, X) dt = 1 - \int_{t=0}^T f_{01}(t, X) dt = 1 - \int_{t=0}^T \int_{\tau=0}^t f_0(\tau, X) f_1(t - \tau, X) d\tau dt \quad (5)$$

225 Similarly for  $f_{012}$ , the pdf of the sum of the time in the first, second and third states, is given by  
 226 Equation 6:

$$227 \quad f_{012}(t, X) = f_{01} * f_2(t, X) = \int_{\tau=0}^t f_{01}(\tau, X) f_2(t - \tau, X) d\tau \quad (6)$$

228

And the probability that a pipe will be in the fourth state or lower (i.e. either in the first, second,  
229  
third or fourth state) at time  $T$  is as presented in Equation 7:

$$230 \quad S_{012}(T, X) = 1 - \int_{t=0}^T \int_{\tau=0}^t f_{01}(\tau, X) f_2(t - \tau, X) d\tau dt \quad (7)$$

231 The probability that a pipe with age  $t$  and having characteristics corresponding to covariates  $X$   
232 will be in the first deterioration state is expressed in our model as (Equation 8):

$$233 \quad P_0(t, X) = S_0(t, X) = e^{-k_0 e^{\beta_0 X} t} \quad (8)$$

234 with:  $k_0, \beta_0$  = model parameters associated with the residence time in the first condition state.  
235

236 The probability that a pipe of age  $t$  and having characteristics  $X$  is in the second, third or fourth  
237 (final) structural state (respectively  $P_1(t, X)$ ,  $P_2(t, X)$  and  $P_3(t, X)$ ) is computed from the survival  
238 functions for cumulative residence times, as given in Equations 9, 10 and 11 respectively.

$$239 \quad P_1(t, X) = S_{01}(t) - S_0(t, X) = \frac{k_0 e^{\beta_0 X} e^{-k_0 e^{\beta_0 X} t} - k_0 e^{\beta_0 X} e^{-k_1 e^{\beta_1 X} t}}{k_1 e^{\beta_1 X} - k_0 e^{\beta_0 X}} \quad (9)$$

$$240 \quad P_2(t, X) = S_{012}(t) - S_{01}(t, X) = \frac{\left( \begin{aligned} &k_0 k_1 k_2 e^{\beta_0 X + \beta_1 X + \beta_2 X} e^{-k_0 e^{\beta_0 X} t} - k_0 k_1^2 e^{\beta_0 X + 2\beta_1 X} e^{-k_0 e^{\beta_0 X} t} \\ &- k_0 k_1 k_2 e^{\beta_0 X + \beta_1 X + \beta_2 X} e^{-k_1 e^{\beta_1 X} t} + k_0^2 k_1 e^{2\beta_0 X + \beta_1 X} e^{-k_1 e^{\beta_1 X} t} \\ &+ k_0 k_1^2 e^{\beta_0 X + 2\beta_1 X} e^{-k_2 e^{\beta_2 X} t} - k_0^2 k_1 e^{2\beta_0 X + \beta_1 X} e^{-k_2 e^{\beta_2 X} t} \end{aligned} \right)}{\left( k_2 e^{\beta_2 X} - k_1 e^{\beta_1 X} \right) \left( k_1 e^{\beta_1 X} - k_0 e^{\beta_0 X} \right) \left( k_2 e^{\beta_2 X} - k_0 e^{\beta_0 X} \right)} \quad (10)$$

$$241 \quad P_3(t, X) = 1 - S_{012}(t) = 1 - \frac{\left( \begin{aligned} &k_1 k_2^2 e^{\beta_1 X + 2\beta_2 X} e^{-k_0 e^{\beta_0 X} t} - k_1^2 k_2 e^{2\beta_1 X + \beta_2 X} e^{-k_0 e^{\beta_0 X} t} \\ &- k_0 k_2^2 e^{\beta_0 X + 2\beta_2 X} e^{-k_1 e^{\beta_1 X} t} + k_0^2 k_2 e^{2\beta_0 X + \beta_2 X} e^{-k_1 e^{\beta_1 X} t} \\ &+ k_0 k_1^2 e^{\beta_0 X + 2\beta_1 X} e^{-k_2 e^{\beta_2 X} t} - k_0^2 k_1 e^{2\beta_0 X + \beta_1 X} e^{-k_2 e^{\beta_2 X} t} \end{aligned} \right)}{\left( k_2 e^{\beta_2 X} - k_1 e^{\beta_1 X} \right) \left( k_2 e^{\beta_2 X} - k_0 e^{\beta_0 X} \right) \left( k_1 e^{\beta_1 X} - k_0 e^{\beta_0 X} \right)} \quad (11)$$

242 where:  $k_1$  and  $\beta_1$  = model parameters associated with the residence time in the second  
243 deterioration state;  $k_2$  and  $\beta_2$  = model parameters associated with the residence time in the third  
244 deterioration state.

245

#### 246 2.2.1.2 Choice of the covariates and verification of the absence of correlations between them

247 Factors (covariates) integrated in the Cox model should not be correlated, thus a correlation test  
248 should be used to identify possible correlations among the factors before constructing the Cox  
249 model. In the work presented here, the Spearman method was used to determine the  
250 correlations among the factors. As described in many handbooks on statistics (e.g., Sheskin  
251 2003), this test measures the degree of association, linear or not, between two variables, even  
252 for those which are ordinal.

253

254 Also, the factors that should be included in  $X$ , the vector of explanatory variables, should be  
255 factors for which extensive data are available on the studied network and for which an impact on  
256 sewer structural deterioration is suspected a priori. The factors selected for the analysis  
257 presented here were those for which data are generally available for Canadian sewer networks  
258 and that could affect the overall performance and structural state of the pipes. These factors are  
259 grouped into two categories: physical and functional factors. The first category includes general  
260 pipe characteristics such as the age, diameter, length, material, and slope, while the last  
261 concerns the type of network. The selected factors have been frequently identified as influential  
262 factors in previous studies (see Table 1). Pipe age is not included in the vector of explanatory  
263 variables since it appears explicitly in the Cox model as the variable  $t$ .

264

265 Finally, the covariates can be quantitative and/or qualitative. However, qualitative variables  
266 require specific coding (as ordinal or binary variables) to include them in the model. In the  
267 present case study, the material type and network type were qualitative variables. In order to

268 evaluate their potential correlations with other factors, and to enable their integration into the  
 269 Cox model, these variables required coding in the form of variable indicators  $X_i$ . The number of  
 270 variable indicators ( $i$ ) varied according to the number of categories included in the same  
 271 variable, with  $i$  equal to one less than the number of categories (Klein and Moeschberger,  
 272 2003). For example, within Network B, sewer type was coded as follows: combined sewer,  $X_1 =$   
 273 1 and  $X_2 = 0$ ; storm water sewer,  $X_1 = 0$  and  $X_2 = 1$ ; and sanitary sewer,  $X_1 = 0$  and  $X_2 = 0$ . All  
 274 codes for the qualitative variables are provided in the supplementary material (Table S-1).

### 276 2.2.1.3 Calibration of the Cox model parameters

277 Before using the model, all of its parameters  $k_0$ ,  $k_1$ ,  $k_2$ ,  $\beta_0$ ,  $\beta_1$  and  $\beta_2$  should be estimated, based  
 278 on the condition state observed during televisual inspections for a sample of representative  
 279 sewer pipes. They remain specific to each sewer system and must be adjusted according to the  
 280 inspection results, but may be subsequently verified using a cross-validation method as carried  
 281 out in Duchesne *et al.* (2013). In the present study, the calibration of the Cox model was  
 282 performed using the maximum likelihood method. This consisted of estimating the values of the  
 283 parameters that maximized the likelihood function given in Equation 12.

$$284 \quad L = \prod_{k \in cd_0}^{n_{cd_0}} P_0(t_k, X_k) \prod_{k \in cd_1}^{n_{cd_1}} P_1(t_k, X_k) \prod_{k \in cd_2}^{n_{cd_2}} P_2(t_k, X_k) \prod_{k \in cd_3}^{n_{cd_3}} P_3(t_k, X_k) \quad (12)$$

285 with  $t_k$  = age of inspected pipe  $k$  (years);  $X_k$  = values of the covariates for inspected pipe  $k$ ;  $cd_j$  =  
 286 all inspected pipes for which the state was equal to  $j$  at inspection at age  $t_k$ ; and  $n_{cd_j}$  = the  
 287 number of pipes in the set  $cd_j$ .

### 289 3.2.1.4 Determination of significance of each covariate

290 In the proposed method, the statistical significance level of parameters in the vectors  $\beta_0$ ,  $\beta_1$  and  
 291  $\beta_2$  is tested with the likelihood ratio test (Thiombiano 2013; Klein and Moeschberger 2003). This

292 test verifies if the coefficients corresponding to the factors integrated into the model (i.e.  
 293 elements of  $\beta_0$ ,  $\beta_1$  and  $\beta_2$ ) are significantly different from zero. It is based on the calculation of  
 294 the distance  $\lambda$  between the logarithm of the likelihood function, calculated with the  $\beta(X_i)$  values  
 295 estimated during calibration (different from zero; unrestricted model,  $\beta_{unr}$ ), and the logarithm of  
 296 the likelihood function calculated with  $\beta(X_i)$ , the value of the coefficient for the analyzed factor,  
 297 forced to zero (restricted model,  $\beta_{res}$ ) (Equation 13).

$$298 \quad \lambda = 2[LL(\beta_{unr}) - LL(\beta_{res})] \quad (13)$$

299  
 300 Under the null hypothesis ( $\beta(X_i) = 0$ ),  $\lambda$  follows a Chi-square law ( $\chi^2_\alpha$ ) with the number of  
 301 degrees of freedom equal to the number of imposed constraints for this hypothesis (number of  
 302 parameters forced to zero) (Thiombiano 2013; Klein and Moeschberger 2003). If the probability  
 303 that the null hypothesis is valid is greater than the chosen significance level ( $\alpha = 0.05$  here),  
 304 then the restricted model is accepted. In the opposite case, the unrestricted model is accepted  
 305 and the coefficient corresponding to the evaluated factor is judged significantly different from  
 306 zero.

### 307 308 *2.2.2 Comparison of results with those of two simpler common methods*

309 The results of the above method for the evaluation of the significant factors, which should be  
 310 considered to predict the structural deterioration of sewer pipes, were compared to those of two  
 311 simpler methods. The first method (Figure 1, method B) involves the separation of pipes into  
 312 cohorts sharing common characteristics, according to the factors analyzed, and the comparison  
 313 of their deterioration curves as computed with the Cox model without covariates. Table 4  
 314 summarizes the cohorts created for this method and their characteristics. Each cohort should  
 315 contain a sufficient number of pipes in order to be able to establish significant statistical  
 316 relationships between the age and structural state; for this reason, the impact of the slope could

317 not be analyzed using this method. Accordingly, for Networks A and B, respectively, twelve and  
318 eleven models were calibrated (one for each cohort). These models were then used to  
319 calculate, for each cohort, the probability of being in each of the considered structural states,  
320 with respect to the age of the pipes. This kind of analysis is similar to those conducted by Ana *et al.*  
321 *al.* (2008), Baur and Herz (2002), Duchesne *et al.* (2013), and Micevski *et al.* (2002).

322

323 ➤ **Insert Table 4 here**

324

325 The second method (Figure 1, method C) involves evaluating whether the distributions of the  
326 values for each factor are significantly different among the structural states. When the  
327 distribution for a given factor differs according to the structural state of the pipes, it can be  
328 considered to be influential to the process of structural deterioration. This evaluation was  
329 performed using the Kruskal-Wallis test statistic (details are found in Sheskin 2003), which  
330 allows for the comparison of the distributions of two or more samples of different sizes.  
331 However, this test can only be used to compare the distributions of quantitative data. Therefore,  
332 only the impacts of age, diameter, length, and slope were evaluated with this method for the  
333 pipes of Network A. This also held true for Network B, except for the slope, which was not  
334 available.

335

### 336 **3. Results and discussion**

#### 337 **3.1 Statistical significance of factors according to the proposed method using the Cox** 338 **model**

339 The first step before applying the Cox model is to evaluate the correlation between covariates.  
340 The results of the Spearman test are presented in Table 5 for Networks A and B.

341

342 ➤ **Insert Table 5 here**

343

344 These results demonstrate that correlations exist between some of the factors for the two  
345 networks. For Network A, the pipe diameter, type of network, and type of material are  
346 correlated. For Network B, correlations are noted between the pipe diameter and network type,  
347 as well as between the pipe diameter and material, and between the material and network type  
348 (combined or other).

349

350 Following these observations, the pertinence of each covariate in the Cox model was first  
351 evaluated for Networks A and B without accounting for the observed correlations (section 3.1.1).  
352 Next, the impacts of these correlations were estimated (section 3.1.2). In both cases and as  
353 mentioned above, the statistical significance of each factor was evaluated using the likelihood  
354 ratio test.

355

### 356 3.1.1 Statistical significance of factors in Cox model without consideration of correlations 357 between covariates

358 In this section, a “global” Cox model that integrates all of the evaluated factors for each network  
359 is established. The statistical significance of the coefficients corresponding to each covariate for  
360 Networks A and B are presented in Table 6 (in this table, for all coefficients,  $p\_value = 1$  and  
361  $\chi^2_{0.05} = 3.84$ ).

362

363 ➤ **Insert Table 6 here**

364

365 As reported in the two tables, none of the coefficients are significantly different from zero  
366 ( $p\_value = 1$ ). This suggests that the factors evaluated for the two networks do not significantly  
367 impact the aging process of the pipes. However, the lack of statistical significance for some of



368 these factors may be the result of correlations among the variables or correlations between the  
369 variables and the age of the pipes (Table 5). The following section describes how these  
370 correlations were taken into account.

371  
372 3.1.2 Statistical significance of factors in Cox model with consideration of correlations between  
373 covariates

374 To evaluate the impacts of the correlations between factors on the results of the likelihood ratio  
375 test, several other models were tested. These models only integrated the covariates that were  
376 not correlated. Thus, based on the previously identified correlations, two models were tested for  
377 Network A. The first model only takes into account the covariates diameter, length, and slope.  
378 The second model incorporates the covariates network type and material, in the place of the  
379 pipe diameter. For Network B, once again based on the previously identified correlations, three  
380 models were created: 1) the first includes the diameter and length; 2) the second includes the  
381 material and length; and 3) the third includes the type of network and the length. Results of the  
382 likelihood ratio tests for the different models for Networks A and B are presented in Table 7 (in  
383 this table, for all coefficients,  $p\_value = 1$  and  $\chi^2_{0.05} = 3.84$ ).

384  
385 ➤ Insert Table 7 here

386  
387 Again, despite the inclusion of correlations between certain covariates of the two networks, the  
388 likelihood ratio test demonstrated that none of the covariates except for age were influential in  
389 the structural deterioration process of the pipes in these networks (all  $p\_values = 1$ ). Similar  
390 results were obtained by testing models that incorporate only the age of the pipes and one  
391 covariate at a time.

392

393 Based on these results, the Cox model without covariates was finally used to estimate the  
394 probability that a pipe will be in structural state 0, 1, 2-3, or 4-5 as a function of its age, for  
395 Networks A and B. Figure 2 shows a comparison of the estimated and observed proportions for  
396 Network A. This figure demonstrates the ability of the model to reproduce the current structural  
397 condition of the pipes. However, this visual comparison provides only a partial estimation of this  
398 ability, because each point represents a different number of pipes. The value of the likelihood  
399 function (used previously in the likelihood ratio test to compare the different models) is a better  
400 estimator of this ability. A slight overestimation of the probabilities for young pipes to be in state  
401 0 can be noted in Figure 2; this is due to the fact that the curve must pass through one for 0-  
402 year-old pipes. In addition, the probabilities for older pipes (61-70 years old) to be in  
403 deterioration states 0, 2-3, and 4-5 seem to be less well estimated than for other pipe ages; this  
404 is due to the limited number of inspected pipes in this age range.

405  
406 ➤ **Insert Figure 2 here**

### 407 408 **3.2 Comparison of structural deterioration of pipes separated according to evaluated** 409 **factors**

410 The results of the Cox model without covariates, separating the pipes into cohorts, are  
411 presented in this section. Because of the large number of cohorts, only the probabilities  
412 associated with the final (4-5) state are presented. Figure 3 illustrates the probabilities that pipes  
413 in Network A will be in state 4-5. Note that in Figure 3b, the probability curves for the two sewer  
414 type cohorts overlap, whereas in Figure 3d, the curves corresponding to the pipes with lengths  
415 less than 60 m and those with lengths between 60 and 120 m also overlap. Results for Network  
416 B are provided in the supplementary material (Figure S-1; in this figure, the probability curves  
417 for the combined and stormwater sewers overlap in panel (b) and the two curves overlap in  
418 panel (d)).

419

420 ➤ **Insert Figure 3 here**

421

422 As shown in Figure 3, the application of the Cox model without covariates to different cohorts of  
423 pipes in Network A demonstrates that the structural conditions of pipes evolve similarly for  
424 different cohorts. Older pipes are more likely to be damaged, regardless of either their physical  
425 or functional characteristics. However, for the majority of factors defining the cohorts, slight  
426 differences can be noted for the probabilities that pipes will be in state 4-5 over time. Most of  
427 these differences are very small. However, the largest differences are found for the type of  
428 material for the pipes in Network A (Figure 3c). Therefore, this factor seems to affect the  
429 deterioration process in the pipes of Network A, a priori. There are also marked differences for  
430 the pipes of Network B, classified by diameter, in relation to the probabilities that the pipes will  
431 be in state 4-5 (Figure S-1a); the diameter can therefore be considered to be a potentially  
432 influential factor for Network B. This evaluation method remains visual, and the results greatly  
433 depend on the amount of data for each cohort used for model calibration.

434

### 435 **3.3 Comparison of distributions of factors classified by structural state**

436 Table 8 presents the results of the Kruskal-Wallis test for comparison of distributions of the  
437 studied factors between very good and poor structural condition states (0 and 4-5) for Networks  
438 A and B. Box diagrams showing the distributions of the factors for Networks A and B for the  
439 same two structural states, 0 and 4-5, are presented in the supplementary material (Figure S-2).

440

441 ➤ **Insert Table 8 here**

442

443 Based on a significance threshold of 0.05, it is evident that the pipe age has a significant impact  
444 on the structural deterioration process for the pipes in the studied networks ( $p\_value < 0.05$ ; the  
445 pipes in state 4-5 tend to be older than those in state 0). This method also highlights the  
446 possible impact that a pipe's slope has on the aging of pipes in Network A; the pipes in state 4-5  
447 have greater mean slopes than those in state 0. However, this method, like the preceding one,  
448 does not take into account the possible correlations among factors. Additionally, the use of this  
449 method is limited by its dependence on the type of available data.

450

### 451 **3.4 Comparison of results obtained from three methods**

452 All of the assessments performed with the Cox model combined with the likelihood ratio test  
453 demonstrate that age alone can explain, in a significant manner, the structural conditions of the  
454 pipes in Networks A and B. The addition of other factors as covariates in the Cox model does  
455 not improve the prediction of the structural states of these pipes over time. This includes factors  
456 that were determined to be influential in the aging process using the two simpler methods.  
457 However, the identification of some influential factors with the simpler methods (e.g.,  
458 the material type for Network A and pipe diameter for Network B) may be the result of  
459 correlations among the variables or correlations between the variables and the age of the pipes  
460 (Table 5). Indeed, the two simpler methods do not take into account the possible correlations  
461 among factors. Additionally, the use of a method that compares the distributions of factors  
462 among the different structural states (using, for example, the Kruskal-Wallis test) cannot take  
463 into account the possible correlation between the age and some influential factors; it is also  
464 limited by its dependence on the type of available data (quantitative and continuous). The Cox  
465 model method presents the following advantages over the two simpler methods: 1) it does not  
466 require grouping data together according to a given characteristic (which in some cases may  
467 reduce the amount of data and thus hamper a statistical analysis), and 2) it can treat both  
468 quantitative and qualitative data.

469  
470 The fact that no significant factors other than age were found using this method, including  
471 factors that are often identified as important by other researchers and other methods (e.g.,  
472 diameter, material, and type of network), may be explained by the amounts and types of data  
473 available to assess the impact of each of these factors. For example, the majority of the  
474 inspected pipes in Networks A and B were made of concrete, which makes it difficult to  
475 effectively evaluate the impact of the type of pipe material on the deterioration process of the  
476 pipes. If one assumes that Networks A and B, and the data that are available to characterize  
477 them, are representative of most Canadian wastewater systems, age would remain the only  
478 significant factor that would need to be taken into account to model the structural deterioration  
479 of these networks.

480

#### 481 **4. Conclusion**

482 In this article, a new robust and standardized method, based on the use of the Cox model, was  
483 proposed to identify the most influential factors, which should be taken into account when  
484 modeling the structural deterioration of sewer pipes. To the best of our knowledge, this was the  
485 first time that the Cox model has been used to model the structural deterioration of sewer pipes.  
486 A calibration method was also proposed to apply the Cox model to the mathematical  
487 representation of a series of successive degradation states. Then, the impacts of physical and  
488 functional factors on the structural deterioration of pipes of two Canadian sewer networks were  
489 assessed using this method and two simpler ones. Results of this comparison highlighted the  
490 importance of evaluating interactions and correlations among factors. Use of simpler methods  
491 for the identification of the significant influential factors should thus be avoided.

492

493 Within the two networks examined here, pipe age was unanimously identified by the three  
494 methods as the main factor influencing sewer pipe deterioration state. This was particularly  
495 evident using the Cox model with covariates, which demonstrated that taking the age alone into  
496 account could provide satisfactory predictions of the structural states of the pipes of the studied  
497 networks. If databases were available that included, for example, information on the structural  
498 states of several pipes made of different types of material and having a wide range of different  
499 ages, the results obtained could have been quite different. However, considering that the  
500 networks studied are representative of Canadian networks, and the data included variables that  
501 are generally available for these networks, it is unlikely that the integration of factors other than  
502 age in structural deterioration models would significantly benefit these networks.

503

504 The model and assessment methods presented in this article are useful tools for planning the  
505 renewal of sewer pipes. In addition to the structural deterioration of the network, this planning  
506 should ideally take into account the evolution of the hydraulic performances of sewers, which is  
507 all the more critical in the context of climate change. Future work should focus on the integration  
508 of these aspects in order to improve renewal procedures, reduce the costs associated with  
509 them, and improve the overall performance of sewage systems.

510

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515

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**Table 1.** Summary of studies that assessed the impacts of different variables on the sewer structural deterioration process

<b>Authors</b>	<b>Model type used</b>	<b>City of application</b>	<b>Evaluated factors</b>	<b>Influential factors</b>	<b>Statistical tests used</b>
Ariaratnam et al. (2001)	Logistic regression	Edmonton * (Canada)	Age; Diameter; Depth; Sewer type; Material; Interactions between factors (Age; Diameter; Sewer type)	Age; Diameter; Sewer type	Wald Likelihood ratio
Micevski et al. (2002)	Markov model	Newcastle ** (Australia)	Diameter; Material; Soil type; Exposure classification (distance from the coastline); Hydraulic performance	Diameter; Material; Soil type; Exposure classification	Chi squared
Baur and Herz (2002)	Cohort survival model	Dresden* (Germany)	Period of construction; Material; Location relative to other infrastructure (road network); Diameter; Slope; Sewer type; Shape; Sewer function (minor or major network)	Material; Period of construction; Location relative to other infrastructure	Visual comparison
Baik et al. (2006)	Markov and ordered probit model	San Diego † (United States)	Age; Length; Diameter; Material; Slope	Age; Length; Diameter; Slope	Measure of overall statistical fit: $\rho^2$ (for ordered probit model)
Tran et al. (2006)	Multiple discriminant analysis regression	Greater Dandenong** (Australia)	Age; Diameter; Depth; Slope; Location (reserve, under road, under nature strip, under easement); Trees root presence; Hydraulic performance; Soil type; Soil moisture index at dry condition	According to the chi squared test and stepwise method: Hydraulic performance According to ANOVA 1: Slope; Depth	ANOVA 1 Stepwise method Univariate Analysis with Chi squared
Chughtai and Zayed (2008)	Multiple regression	Niagara Falls and Pierrefonds (Canada)	Age; Diameter; Length; Material; Depth; Slope; Bedding factor; Location relative to other infrastructure (road network category); Interactions between some factors	Concrete pipes: Age associated to material; Location relative to other infrastructure; Depth; Depth associated to Bedding factor; Bedding factor Asbestos cement pipes: Age; Depth	t-test

Authors	Model type used	City of application	Evaluated factors	Influential factors	Statistical tests used
				associated to Length PVC pipes: Age; Length; Location relative to other infrastructure; Bedding factor; Diameter associated to Depth	
Ana et al. (2008)	Cohort survival model	Leuven* (Belgium)	Period of construction; Diameter; Length; Shape; Material; Slope; Depth; Location relative to other infrastructure (road network)	Period of construction; Length; Material	Visual comparison
Ana et al. (2009)	Multiple logistic regression	Leuven* (Belgium)	Age; Period of construction; Diameter; Length; Shape; Material; Sewer type; Slope; Depth; Location relative to other infrastructure (road network)	Age; Material; Length	Wald Likelihood ratio
Younis and Knight (2010)	Ordinal regression	Niagara Falls † (Canada)	Age; Material; Interaction between Age and Material	Material; Interaction between Age and Material (Age is influent only for concrete pipes)	Wald
Salman and Salem (2012)	Multinomial logistic regression	Cincinnati †† (United States)	Age; Diameter; Length; Material; Sewer type; Slope; Location relative to other infrastructure (road network); Depth; Interactions between factors: 27 two-way interactions (except interaction between Sewer type and Material) and 5 three-way interactions	Age; Diameter; Length; Material; Sewer type; Slope; Location relative to other infrastructure; Depth; 13 two-way interactions and 2 three-way interactions	Wald (for factors) Stepwise method (for interactions between factors)

Authors	Model type used	City of application	Evaluated factors	Influential factors	Statistical tests used
	Binary logistic regression		Age; Diameter; Length; Material; Sewer type; Slope; Location relative to other infrastructure (Road network); Depth; Interactions between factors: 20 two-way interactions (except interaction between Sewer type and Material)	Age; Diameter; Length; Slope; Material; Sewer type; Location relative to other infrastructure; Depth; 7 two-way interactions	Wald (for factors) Stepwise method and Likelihood ratio (for interactions between factors)
Ugarelli et al. (2013)	GompitZ deterioration modeling tool using the combination of Markov and survival model	Oslo †† (Norway)	Diameter; Sewer type; Period of construction; Road traffic; Soil type; Tramway proximity; Trees presence; Interactions between factors	For concrete pipes up to 600 mm: Diameter; Sewer type; Period of construction; Soil type; Trees presence; Interaction between Sewer type and trees presence	Likelihood ratio
Rokstad and Ugarelli (2015)	GompitZ deterioration modeling tool (non-homogeneous Markov Chain model)	Oslo * (Norway)	Diameter; Sewer type; Period of construction; Road traffic; Bedding factor; Trees presence	For all pipes material : Diameter; Sewer type; Period of construction; Road traffic; Bedding factor; Trees presence	Chi squared
Fuchs-Hanusch et al. (2015)	Logistic regression models	Unnamed * (Austria)	Material; Vintage (period of construction); Sewage type; Profile type; Width; Height; Length; Depth	Material; Length; Width; Vintage; Profile type	Likelihood ratio
Tscheikner-Gratl et al. (2014)	Binary logistic regression	Alpine city ††† (Austria)	Age; Diameter; Material Slope; Length; Shape	Age; Diameter; Slope; Length; Shape	Wald

\* : Total network: sanitary, stormwater and combined sewers

\*\* : Stormwater sewer

† : Sanitary sewer

†† : Sanitary and combined sewers

††† : Combined sewer

**Table 2.** Characteristics of sewer pipes in Networks A and B

Installation year	Network A		Network B	
	Whole network (%)	Inspected pipes (%)	Whole network (%)	Inspected pipes (%)
1900-1929	5.0	7.0	0.7	0.0
1930-1949	6.1	8.1	1.7	1.7
1950-1969	25.4	32.5	44.32	55.6
1970-1989	36.6	36.9	33.9	35.7
1990-2012	21.8	13.6	19.2	6.8
Unknown	4.8	1.6	0.0	0.0
Sewer type	Total length (km)	Inspected length (km)	Total length (km)	Inspected length (km)
Combined	586	383	442	70
Stormwater	1,972	53	424	43
Sanitary	2,067	1,213	420	46
Diameter	Total length (km)	Inspected length (km)	Total length (km)	Inspected length (km)
≤ 300 mm	2,252	1,209	426	23
300 - 600 mm	1,236	344	570	81
> 600 mm	636	93	290	52
Unknown	501	3	-	-
Material	Total length (km)	Inspected length (km)	Total length (km)	Inspected length (km)
PVC	471	273	-	-
Reinforced concrete	2,167	1,212	-	-
Concrete	-	-	911	120
Other	267	115	361	39
Unknown	1,720	49	14	-

**Table 3.** Proportion of pipes in each structural state in different age ranges for Networks A and

B

<b>Network A</b>				
<b>Age (years)</b>	<b>Proportion of pipes in state 0 (%)</b>	<b>Proportion of pipes in state 1 (%)</b>	<b>Proportion of pipes in state 2-3 (%)</b>	<b>Proportion of pipes in state 4-5 (%)</b>
0 to 10	83	9	7	2
11 to 20	80	9	10	1
21 to 30	71	14	13	1
31 to 40	66	15	16	3
41 to 50	57	20	19	4
51 to 60	45	24	25	5
<b>Network B</b>				
<b>Age (years)</b>	<b>Proportion of pipes in state 0 (%)</b>	<b>Proportion of pipes in state 1 (%)</b>	<b>Proportion of pipes in state 2-3 (%)</b>	<b>Proportion of pipes in state 4-5 (%)</b>
0 to 10	54	15	27	4
11 to 20	47	24	29	0
21 to 30	41	18	40	1
31 to 40	37	15	47	1
41 to 50	28	13	58	2
51 to 60	14	14	70	1
61 to 70	0	0	100	0

**Table 4.** Descriptions of analyzed cohorts for the two sewer networks

Factors	Network A		Network B	
	Cohort name	Description	Cohort name	Description
<b>All factors</b>	A1	All inspected pipes ( $n = 16,896$ )	B1	All inspected pipes ( $n = 2,380$ )
<b>Diameter</b>	A2	Inspected pipes with diameter < 300 mm ( $n = 13,209$ )	B2	Inspected pipes with diameter < 300 mm ( $n = 353$ )
	A3	Inspected pipes with diameter between 300 and 600 mm ( $n = 3,007$ )	B3	Inspected pipes with diameter between 300 and 600 mm ( $n = 1,336$ )
	A4	Inspected pipes with diameter $\geq 600$ mm ( $n = 679$ )	B4	Inspected pipes with diameter $\geq 600$ mm ( $n = 691$ )
	A5	Inspected sanitary pipes ( $n = 13,544$ )	B5	Inspected combined pipes ( $n = 1,055$ )
<b>Sewer type</b>	A6	Inspected combined and stormwater pipes ( $n = 3,352$ )	B6	Inspected stormwater pipes ( $n = 637$ )
			B7	Inspected sanitary pipes ( $n = 688$ )
	A7	Inspected PVC pipes ( $n = 2,882$ )	B8	Inspected concrete pipes ( $n = 1,808$ )
<b>Material</b>	A8	Inspected reinforced concrete pipes ( $n = 12,900$ )	B9	Inspected pipes of all other materials (asbestos cement, vitrified clay, PVC, and cast iron) ( $n = 572$ )
	A9	Inspected pipes of all other materials (asbestos cement, non-reinforced concrete, corrugated steel, brick, cast iron, pipe reinforced with glass fiber, polyethylene, steel) ( $n = 1,114$ )		
<b>Length</b>	A10	Inspected pipes with length < 60 m ( $n = 6,938$ )	B10	Inspected pipes with length < 60 m ( $n = 1,172$ )
	A11	Inspected pipes with length between 60 and 120 m ( $n = 9,221$ )	B11	Inspected pipes with length $\geq 60$ m ( $n = 1,208$ )
	A12	Inspected pipes with length $\geq 120$ m ( $n = 737$ )		



**Table 5.** Spearman correlation coefficients for pipe age and other covariates

<b>Network A</b>							
	<b>Age</b>	<b>Diameter</b>	<b>Length</b>	<b>Sewer type <math>X_1</math></b>	<b>Slope</b>	<b>Material <math>X_1</math></b>	<b>Material <math>X_2</math></b>
<b>Age</b>	1	0.24	0.06	0.37	0.01	0.14	-0.19
<b>Diameter</b>		1	0.02	0.49	0.07	0.30	-0.29
<b>Length</b>			1	-0.06	0.01	0.08	-0.09
<b>Sewer type <math>X_1</math></b>				1	0.18	0.07	-0.06
<b>Slope</b>					1	-0.10	0.15
<b>Material <math>X_1</math></b>						1	-0.81
<b>Material <math>X_2</math></b>							1

<b>Network B</b>						
	<b>Age</b>	<b>Diameter</b>	<b>Length</b>	<b>Material <math>X_1</math></b>	<b>Sewer type <math>X_1</math></b>	<b>Sewer type <math>X_2</math></b>
<b>Age</b>	1	0.13	0.10	0.24	0.41	-0.27
<b>Diameter</b>		1	0.14	0.53	0.28	0.23
<b>Length</b>			1	-0.01	0.02	-0.02
<b>Material <math>X_1</math></b>				1	0.47	0.05
<b>Sewer type <math>X_1</math></b>					1	-0.54
<b>Sewer type <math>X_2</math></b>						1



**Table 6.** Results of likelihood ratio test for the global Cox models

<b>Network A</b>																		
<b>Coefficients for the global model</b>																		
	<b>Diameter</b>			<b>Length</b>			<b>Sewer type <math>X_1</math></b>			<b>Slope</b>			<b>Material <math>X_1</math></b>			<b>Material <math>X_2</math></b>		
<b>Estimated parameters</b>	$\beta_0$	$\beta_1$	$\beta_2$	$\beta_0$	$\beta_1$	$\beta_2$	$\beta_0$	$\beta_1$	$\beta_2$	$\beta_0$	$\beta_1$	$\beta_2$	$\beta_0$	$\beta_1$	$\beta_2$	$\beta_0$	$\beta_1$	$\beta_2$
	0.00	0.14	0.00	0.07	0.00	0.03	0.47	0.00	0.00	0.08	0.00	0.41	0.27	0.00	0.00	0.01	0.51	0.88
$\lambda$	-32.90			-30.58			-333.71			-4.77			-303.74			-150.02		
<b>Network B</b>																		
<b>Coefficients for the global model</b>																		
	<b>Diameter</b>			<b>Length</b>			<b>Material <math>X_1</math></b>			<b>Sewer <math>X_1</math></b>			<b>Sewer <math>X_2</math></b>					
<b>Estimated parameters</b>	$\beta_0$	$\beta_1$	$\beta_2$	$\beta_0$	$\beta_1$	$\beta_2$	$\beta_0$	$\beta_1$	$\beta_2$	$\beta_0$	$\beta_1$	$\beta_2$	$\beta_0$	$\beta_1$	$\beta_2$			
	0.00	0.00	0.02	0.00	0.07	0.01	0.06	0.00	0.00	0.17	0.13	0.04	0.13	0.65	0.01			
$\lambda$	0.00			-2.32			-4.54			-26.69			-54.77					

**Table 7.** Results of likelihood ratio test for the simplified models

<b>Network A</b>															
<b>Model 1</b>															
	<b>Diameter</b>			<b>Length</b>			<b>Slope</b>								
<b>Estimated parameters</b>	$\beta_0$	$\beta_1$	$\beta_2$	$\beta_0$	$\beta_1$	$\beta_2$	$\beta_0$	$\beta_1$	$\beta_2$						
	0.08	0.03	0.03	0.49	0.00	0.00	0.03	0.01	0.56						
$\lambda$	-43.77			-38.53			-12.95								
<b>Model 2</b>															
	<b>Length</b>			<b>Sewer type <math>X_1</math></b>			<b>Slope</b>			<b>Material <math>X_1</math></b>			<b>Material <math>X_2</math></b>		
<b>Estimated parameters</b>	$\beta_0$	$\beta_1$	$\beta_2$	$\beta_0$	$\beta_1$	$\beta_2$	$\beta_0$	$\beta_1$	$\beta_2$	$\beta_0$	$\beta_1$	$\beta_2$	$\beta_0$	$\beta_1$	$\beta_2$
	0.06	0.01	0.02	0.48	0.00	0.02	0.04	0.00	0.28	0.22	0.00	0.08	0.02	0.38	0.87
$\lambda$	-22.67			-346.56			-0.08			-207.43			-143.32		
<b>Network B</b>															
<b>Model 1</b>															
	<b>Diameter</b>			<b>Length</b>											
<b>Estimated parameters</b>	$\beta_0$	$\beta_1$	$\beta_2$	$\beta_0$	$\beta_1$	$\beta_2$									
	0.02	0.02	0.02	0.00	0.06	0.00									
$\lambda$	-0.87			-1.93											
<b>Model 2</b>															
	<b>Length</b>			<b>Sewer type <math>X_1</math></b>			<b>Sewer type <math>X_2</math></b>								
<b>Estimated parameters</b>	$\beta_0$	$\beta_1$	$\beta_2$	$\beta_0$	$\beta_1$	$\beta_2$	$\beta_0$	$\beta_1$	$\beta_2$						
	0.00	0.08	0.00	0.20	0.14	0.00	0.15	0.65	0.00						
$\lambda$	-2.60			-34.71			-58.03								
<b>Model 3</b>															
	<b>Length</b>			<b>Material <math>X_1</math></b>											
<b>Estimated parameters</b>	$\beta_0$	$\beta_1$	$\beta_2$	$\beta_0$	$\beta_1$	$\beta_2$									
	0.00	0.07	0.00	0.19	0.00	0.00									
$\lambda$	-2.54			-40.09											

**Table 8.** Results of Kruskal-Wallis test for comparison between very good and poor structural condition states

Network	p_value for the compared factors			
	Age	Diameter	Length	Slope
A	1.241E-38	0.215	0.094	0.013
B	0.007	0.965	0.931	

## Figure captions

**Figure 1.** Illustration of methodology

**Figure 2.** Proportion of pipes assigned to structural states 0 (very good), 1 (good), 2-3 (fair), and 4-5 (poor) for Network A

**Figure 3.** Probability that pipes of Network A will be in state 4-5 (poor structural state), classified by (a) diameter, (b) sewer type, (c) material and (d) length

1. Preparation of the database



2. Selection of the factors to take into account



3. Identification of the influential factors



A. With the proposed method based on the Cox model



A-i. Verification of the absence of correlation between the covariates (Spearman test)



A-ii. Calibration of the Cox model with covariates



A-iii. Application of the likelihood ratio test

B. Using the model without covariates and separating the pipes in cohorts (repeat B-i to B-iii for each factor)



B-i. Separation of the inspected pipes into cohorts



B-ii. Calibration of the Cox model without covariates



B-iii. Visual comparison of the simulated curves

C. By comparing their distributions for different condition states (repeat C-i to C-ii for each factor)



C-i. Separation of the inspected pipes by condition classes



C-ii. Comparison of the distributions of the factors for different condition classes (Kruskal-Wallis test)



4. Comparison of the results

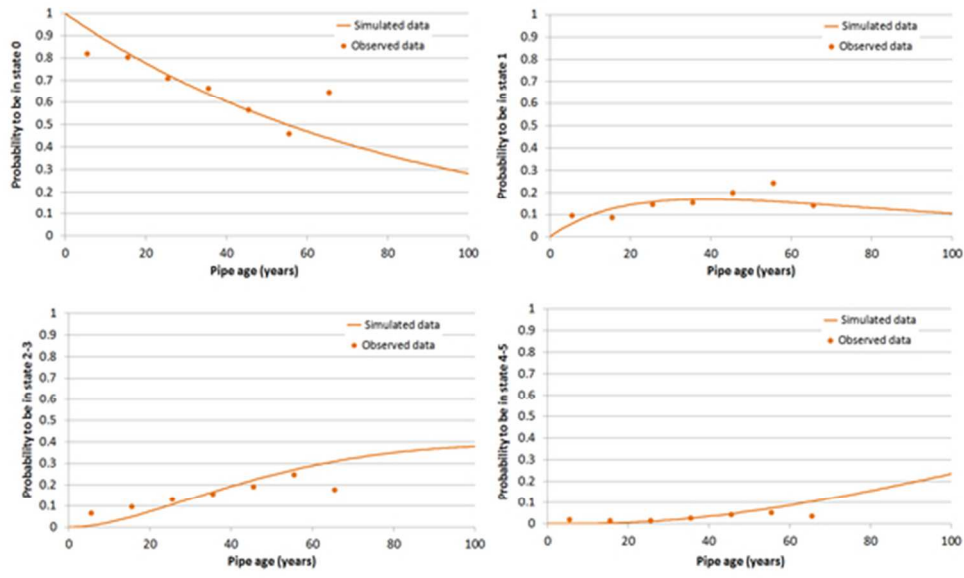


Figure 2. Proportion of pipes assigned to structural states 0 (very good), 1 (good), 2-3 (fair), and 4-5 (poor) for Network A.

50x30mm (300 x 300 DPI)



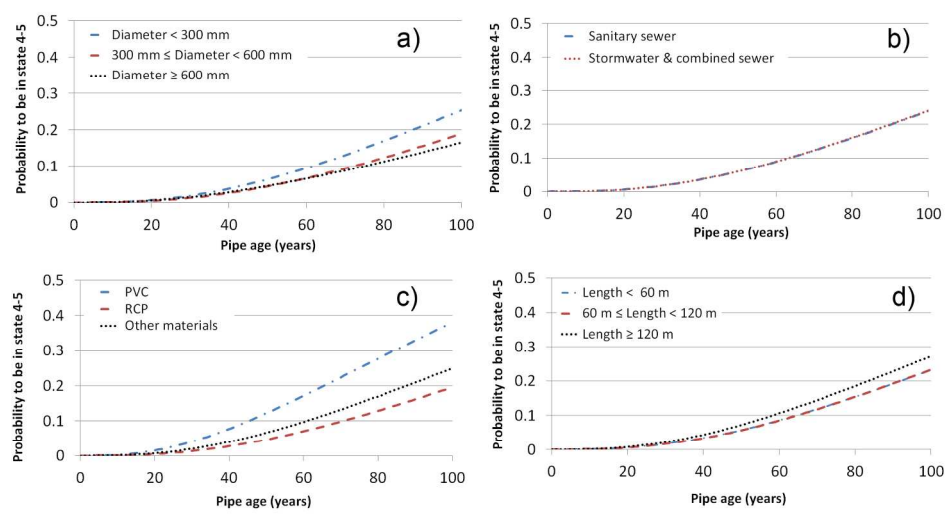


Figure 3. Probability that pipes of Network A will be in state 4-5 (poor structural state), classified by (a) diameter, (b) sewer type, (c) material and (d) length

186x101mm (300 x 300 DPI)